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**A NEW APPROACH FOR WATER ALLOCATION  
SCHEDULING IN IRRIGATION OPEN CHANNELS  
CASE STUDY: ALMANNA MAIN CANAL, AND ITS BRANCHES**

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**Abstract**

In Egypt, the Agriculture sector consumes about 85.90% of the total actual consumption of water. The irrigation canals network in Egypt at all levels suffer from water losses, distorted sections, and un-equitable distribution of irrigation water among beneficiaries. With population growth, expansion of economic and industrial activities and entry of Egypt into the stage of water poverty, rationalization of the consumption of irrigation water has become an urgent requirement and a key factor in achieving sustainable water development. Although crops may need different water quantities during their growth stages, irrigation canals are constantly supplied with almost the same monthly discharge during periods of peak or minimum requirements. The objective of this study is to activate a new approach for the reasonable management of the irrigation network for the study area. This approach allows the canals network to be supplied with monthly discharges consistent with the actual water consumption of the cultivated plant. Also, it helps in rationalizing consumption during the agricultural season. The new approach was applied to Almanna main canal and its branches which belong to the Assiut governorate in Egypt, as an example of the irrigation system in Egypt. Rotational distribution is practiced at the distributary canal level. The results

indicated that applying the new approach on Almana distributary canals can save a large amount of irrigation water that reaches about 19.375 Million cubic meters monthly, representing 48.68% of all irrigation water given to Almana distributary canals. Also, the percentage value of the new discharges of Almana distributary canals, which calculated according to the water needs of crops cultivated in the area, ranged between 32 and 67% of the designed discharges.

**Keywords:** Water Consumption; Crop Water Requirements; Water Allocation.

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## 1. Introduction

The growing demand for water has ushered in the need for effective use of water in the irrigation sector with different methods of management. The main aim of these methods is to meet the crop demand with the available water to get maximum production. Scheduling of water delivery is one among them and it is a core activity that has more influence on the performance of the system compared to other irrigation activities [1]. Scheduling of an irrigation water distribution system should be based on the objectives or targets of the irrigation system that can be measured with performance indicators. The concepts and definitions of performance indicators that describe the quality of irrigation service provided by the managers of the water delivery system have been presented by many researchers such as given by [2, 3, 4, 5, 6 &7]. It is also realized that there are complexities in identifying the objectives, defining them, and assessing them at different levels of an irrigation system by different interest groups with differing perspectives. The complexities are well described in the literature [1& 8].

Today, equity, adequacy, and timeliness are the main objectives of canal water distribution procedures. These operational objectives may be conflicting with or contributing to each other in different magnitudes depending upon the water availability [9]. The canal was designed as continuous water supply systems. The increase in cropping area and changes in cropping pattern in course of time increase the demand in these systems. So, the main canal capacity is inadequate to run all the distributary canals simultaneously. Rotational water

distribution has been introduced in some of the systems to manage the shortage of water.

In most of the irrigation systems, the manager who prepares the irrigation schedule relies mainly on his experience or rules of thumb. He distributes the water to the canals according to the pre-specified water duty. This water duty does not vary with time. It gives an approximate estimation of water required for a virtual cropping pattern over a gross period like a season. If the canals are large in number and vary in design discharge, length and served area, the manager requires special skill to set water allocating priorities for the defined objectives, develop and implement an irrigation schedule under these complex situations. Several models have been developed for irrigation scheduling with optimization and simulation techniques [10, 11, 12&13]. A procedure for the operation of canals using a water balance equation have been developed by Rajput and Michael [14], for the estimation of daily soil moisture status taking a hypothetical case of four branch canal system. This model can be applied to real situations only if the number of branch canals in the network is in multiples of four. Vedula et al. [15] used dynamic programming to develop an irrigation scheduling model for optimal allocation of water during different periods of the season for a single crop. The model considered the soil moisture contribution for estimating the irrigation requirement. Also, Kalu et al. [16] used compromise programming to select the best water distribution policies according to crop yield, efficiency, and equity measures. The model was used in a small command area and emphasis was put on crop production in relation to the water supply. An adjusted irrigation plan proposed for water distribution in large areas by Kan and Shu [17] through lengthening the rotational irrigation periods in no water shortage conditions. This plan allowed good water distribution but it was much limited when under large stresses water supplies considerably decreased. Water distribution hence became a bottleneck for both water supply system and farmers. Farmers are under pressure to grow more, so there is an urgent need to find ways to grow more with less water [18]. A multi-criteria mathematical model for canal irrigation scheduling in rotational distribution was formulated to achieve the objectives of minimizing gate operations, improving the performance of equity, adequacy, and timeliness [19]. Yuanhua and Honyuan [20] evolved a model for canal scheduling with rotational water distribution by estimating the initial soil moisture daily through

the water balance equation and forecasting the weather data and subsequently the irrigation date and depth. Similar observations are discussed by Hill and Allen [21] while developing an irrigation scheduling calendar in Pakistan. Also, the application of the  $0 \pm 1$  linear programming model proposed by Zhi et al. [22] for outlet schedule is limited to irrigation systems where the distribution outlets along the canal (be it main, lateral, tertiary) have the same discharge capacity and such systems are hypothetical. Most of these models have difficulties in field applications for the following reasons: (a) the assumptions made and or the pre-defined mathematical structure involved in developing the optimization problem do not match with the real conditions of the field, and (b) the field measurement data required for these models such as the soil moisture status or plant stress are generally not collected and used in most of the irrigation systems in many countries.

Tests to reduce the non-flexibility in matching irrigation intervals and crop or soil condition are being carried out in Egypt but are hampered by the fact that the cropping pattern is not known, and water flows/volumes cannot be properly measured and regulated [23]. In practice, this means that hopes for a better match between irrigation supplies and demand remain futile until a more accurate regulation of the available volumes is possible, provided the water is available. Rather than the lack of awareness about the need to develop the capacity to deal with the probable future droughts still exists.

Irrigation canals network in Egypt at all levels suffer from water losses, distorted sections, and un-equitable distribution of irrigation water among beneficiaries. With population growth, expansion of economic and industrial activities and entry of Egypt into the stage of water poverty, rationalization of the consumption of irrigation water has become an urgent requirement and a key factor in achieving sustainable water development. Although crops may need different water quantities during their growth stages, irrigation canals are constantly supplied with almost the same monthly discharge during periods of peak or minimum requirements. The objective of this study is to activate a new approach for the reasonable management of the irrigation network for the study area. This approach allows the canals network to be supplied with monthly discharges consistent with the actual water consumption of the cultivated plant. Also, it helps in rationalizing consumption during the agricultural season. The new

approach was applied to Almanna main canal and its branches which belong to the Assiut governorate in Egypt, as an example of the irrigation system in Egypt. Rotational distribution is practiced at the distributary canal level.

## 2. Materials and Methods

### 2.1 Description of the study area

Almanna canal was chosen to conduct the present field study as a representative open channel having specific properties from different technical points of view, soil type, weather condition, and the length with its off-taking canals. Almanna canal belongs to Abnoub Irrigation Engineering Administration in Assiut. The length of Almanna canal is about 32.80 km and its intake located at km 157 at the right bank for the Eastern Naga Hammady canal as shown in Fig. (1). Twenty distributary canals are branched from Almanna canal on both sides as shown in Fig. (2). Total length of the branches is about 47.097 km. Table (1) shows geometrical characteristics and discharges of the different sections of Almanna canal. It was firstly constructed between Al-Matmar and Al-Maabda to carry fresh water for municipal water supply. It has four regulators constructed along its total length. The first one is a head regulator at km (0.000), the second regulator at km. (10.450), the third one at km. (16.450), and the fourth regulator at km. (23.060). Table (2) presents the geometric and hydraulic characteristics of Almanna off-taking canals that can be used to assess and propose the water management criteria. Manning equation is used for calculating the designed discharges of canals as given in Tables (1), and (2). Manning equation in the metric system could be written as:

$$Q = \frac{1}{n} \times S^{0.5} \times A \times R^{2/3} \dots\dots\dots (1)$$

where Q: water discharge (m<sup>3</sup>/s), S: friction slope could be substituted by water slope as demonstrated in Tables (1 and 2) (m/m), R: hydraulic radius (m), and A: area of hydraulic section (m<sup>2</sup>).

The study area is located between 27°16' and 27°33' north Latitude and between 31°27' and 31°01' east Longitude. The area is characterized by arid climate. Tables (3 and 4) show statistic models for the

meteorological data of Abnoub cropped area during a period of 5 years started from 2014 to 2019 as documented in the Arab Alawlamer weather station [24]. The agricultural land is at 50 meters above the mean sea water level. Canal cropped area is 13500 feddans. Winter crops are Wheat, Clover, Bean, while summer ones are Yellow corn, Sorghum, and Basil. Table (5) shows the areas of different crops cultivated during the last 5 years. This table was taken from the Agriculture Directorate in Assiut Governorate. It is noticed from this table that, the areas of cultivated lands differ during the five years, and this may be due to economic, social, or agricultural reasons of the beneficiaries that require the un-constant of the cultivated area. Consequently, there are wastelands that have not been cultivated, so they were not considered when calculating the actual cultivated area. These data were used to calculate the percentage of cultivation of these crops. Through these ratios, the area of cultivation of each crop was calculated as shown in Table (6). By knowing the water requirements of each crop, it is possible to predict the optimum discharge of all canals in the study area.

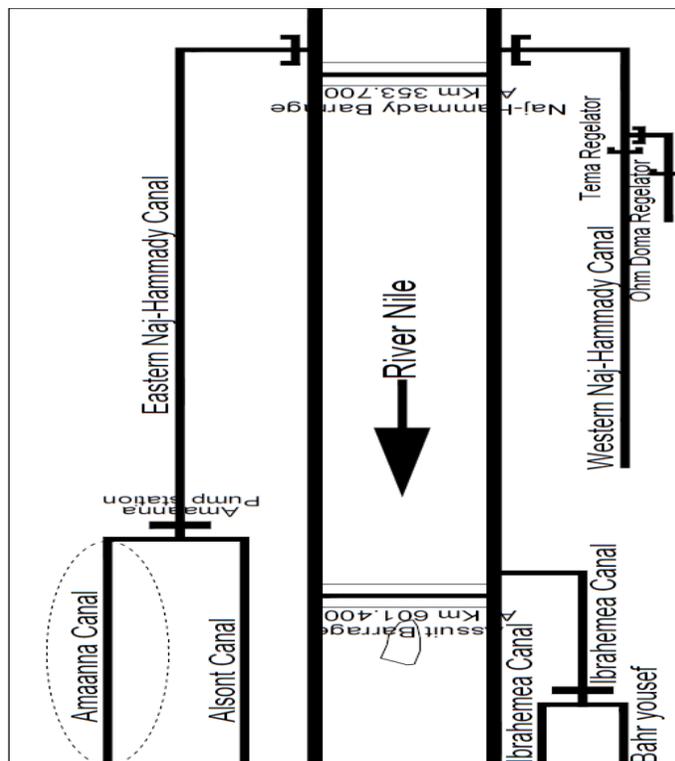


Fig. 1. Synoptic of regional canals' network



**Table 1.** Geometrical characteristics and discharges of the different section of Almanna canal.

No	Sections (Km)	Gate	Area (Fedd)	Length (Km)	Side slope	longitudinal water slope (-)	Bed width (m)	Levels (m)		Q <sub>Designe</sub> (m <sup>3</sup> /sec)
								Bed	Water	
1	(0.000 – 10.45)	Intake regulator-Arab Meter	0.000	10.450	3:2	0.00004	10.00	49.00	51.25	19.34
2	(10.45 – 16.45)	Arab Meter-Shew	7450	6.0000	3:2	0.00005	7.50	48.30	51.00	18.55
3	(16.45 – 23.06)	Shew- Algabrawe	3280	6.6100	3:2	0.00005	6.00	48.00	50.40	14.65
4	(23.06 – 26.40)	Algabrawe - End	2600	3.3400	3:2	0.00010	4.00	47.67	49.73	8.23
5	(26.40 – 32.80)	End	170.0	6.4000	3:2	0.00005	3.00	47.25	49.42	7.55

**Table 2.** Geometrical characteristics and discharges of Almanna off-taking canals

No	Canal	Feeding canal	Intake at Km	Bank	Area (fedd)	Length (Km)	Side Slope	longitudinal water slope (-)	Bed width (m)	Levels (m)		Q <sub>Designe</sub> (m <sup>3</sup> /sec)
										Bed	Water	
1	R. Almanna s. canal	Almanna	10.50	Right	1000	5.827	1:1	0.00010	2.0	49.60	50.80	2.05
2	Al Hammam	Almanna	10.50	Left	3400	9.000	1:1	0.00005	4.0	49.00	50.60	4.39
2-1	Arab meter branch	Al Hammam	1.000	Left	600	2.000	1:1	0.00005	2.0	49.30	50.50	1.45
2-2	Al Atawla	Al Hammam	3.000	Left	500	2.400	1:1	0.00010	1.5	49.20	50.40	1.65
2-3	Al Zafran	Al Hammam	6.30	Left	400	2.300	1:1	0.00005	1.5	49.00	50.29	1.34
2-4	Al Taweel	Al Hammam	6.940	Left	500	2.400	1:1	0.00010	1.5	48.50	50.10	2.92
3	Shew	Almanna	12.50	Right	450	2.300	1:1	0.00010	1.0	49.00	50.20	1.26
4	Western Al Awamer	Almanna	16.30	Left	200	0.800	1:1	0.00005	1.0	48.80	50.28	1.38
5	Eastern Alawamer	Almanna	16.30	Left	400	0.670	1:1	0.00005	1.0	48.75	50.00	0.94
6	Northern Alawamer	Almanna	17.80	Left	780	1.400	1:1	0.00005	1.5	49.00	50.55	2.05
7	Der Shew	Almanna	19.80	Left	400	1.000	1:1	0.00005	1.0	48.55	49.45	0.50
8	Eastern Al kadadeeh	Almanna	21.80	Left	400	1.000	1:1	0.00005	1.0	48.50	49.60	0.74
9	Al Hager	Almanna	22.80	Right	500	4.200	1:1	0.00010	1.5	48.20	49.10	0.95
10	Bani Ibraheem	Almanna	23.80	Left	800	1.400	1:1	0.00005	2.0	48.00	49.65	1.74
10-1	Bani Ibraheem bran.	B. Ibraheem	0.250	Left	400	1.350	1:1	0.00005	1.0	48.00	49.20	1.45
11	Southern Al Gabrawe	Almanna	23.80	Left	500	2.200	1:1	0.00010	1.0	48.00	49.50	2.01
12	No.n Al Gabrawe	Almanna	25.65	Left	400	1.350	1:1	0.00005	1.0	48.00	49.15	0.81
13	Alswalem	Almanna	26.40	Left	700	1.800	1:1	0.00010	1.5	47.70	49.00	1.93
14	Alam Aldeen	Almanna	27.60	Left	500	1.600	1:1	0.00005	1.0	47.49	48.70	0.90
15	Al Haraga	Almanna	28.35	Left	500	2.100	1:1	0.00010	1.0	47.60	48.75	1.15

**Table 3.** Average monthly meteorological data of Abnoub area during winter growing seasons [24].

<b>Season of 2013/2014</b>							
Month	Temperature (C°)		Relative humidity %	E pan mm/day	Wind Speed Km/h	Solar radiation MJ/m <sup>2</sup> /day	No. of sunny hours / day
	Max	Min					
December	23.2	8.50	48.40	5.67	14.40	14.80	9.00
January	22.4	6.30	55.20	2.82	07.90	15.40	8.90
February	23.8	7.40	49.20	3.15	12.00	18.70	9.70
March	27.9	12.1	42.40	4.43	11.50	21.70	9.90
April	32.2	15.8	34.90	6.41	10.92	24.50	10.3
<b>Season of 2014/2015</b>							
December	20.4	7.20	63.20	3.11	16.80	14.80	9.0
January	20.5	5.50	44.00	2.32	16.00	15.40	8.9
February	22.7	7.60	38.80	3.08	16.40	18.70	9.7
March	27.2	12.2	34.00	3.79	19.40	21.70	9.9
April	29.3	14.6	25.60	6.33	19.90	24.50	10.3
<b>Season of 2015/2016</b>							
December	19.50	6.30	59.70	2.50	16.80	14.80	9.0
January	19.00	5.10	60.30	2.17	15.20	15.40	8.9
February	24.50	8.30	50.70	2.74	14.50	18.70	9.7
March	28.00	13.10	41.00	3.99	17.00	21.70	9.9
April	35.10	17.10	31.50	6.40	17.00	24.50	10.3
<b>Season of 2016/2017</b>							
December	23.20	9.00	58.80	5.67	14.60	14.80	9.00
January	19.30	5.30	55.30	2.82	14.80	15.40	8.90
February	20.50	6.30	52.60	3.15	14.50	18.70	9.70
March	25.30	11.00	42.50	4.43	17.20	21.70	9.90
April	31.30	15.50	36.60	6.41	17.30	24.50	10.3
<b>Season of 2017/2018</b>							
December	20.80	8.00	62.80	3.11	16.30	14.80	9.0
January	19.90	6.50	57.50	2.32	15.30	15.40	8.9
February	26.10	11.20	44.30	3.08	14.40	18.70	9.7
March	30.50	14.20	36.20	3.79	16.90	21.70	9.9
April	32.40	16.60	36.20	6.33	18.40	24.50	10.3
<b>Season of 2019/2020</b>							
December	23.20	9.00	58.80	2.50	14.60	14.80	9.0
January	19.30	5.80	52.80	2.18	13.90	15.40	8.9
February	21.80	7.60	51.40	2.75	17.30	18.70	9.7
March	24.70	9.90	42.90	4.01	19.80	21.70	9.9
April	29.60	14.00	36.50	6.22	21.30	24.50	10.3

**Table 4.** Average monthly meteorological data of Abnoub area during summer growing seasons [24]:

<b>Season of 2014</b>							
Month	Temperature (C°)		Relative humidity %	E pan mm/day	Wind Speed Km/h	Solar radiation MJ/m <sup>2</sup> /day	No. of sunny hours / day
	Min	Max					
May	35.60	19.70	35.10	6.52	10.70	27.00	11.40
June	37.80	22.30	33.20	6.61	12.40	28.50	12.30
July	38.30	23.60	32.00	6.70	12.80	28.20	12.20
August	38.40	23.90	33.80	7.10	14.40	27.10	11.90
September	35.80	22.10	33.60	5.00	15.80	23.70	10.80
October	31.30	16.90	36.70	4.60	9.80	19.90	10.00
November	26.60	12.30	45.00	4.45	11.80	16.50	9.40
<b>Season of 2015</b>							
May	35.50	19.70	27.10	6.34	17.40	27.00	11.40
June	36.60	21.30	37.40	6.39	13.00	28.50	12.30
July	38.80	22.80	35.90	6.78	8.20	28.20	12.20
August	40.30	24.80	38.60	7.44	8.40	27.10	11.90
September	38.50	23.80	38.50	5.89	14.60	23.70	10.80
October	33.00	19.50	51.30	4.89	16.30	19.90	10.00
November	26.30	13.20	59.50	4.51	15.20	16.50	9.40
<b>Season of 2016</b>							
May	36.10	20.00	27.70	6.48	20.30	27.00	11.40
June	40.70	24.60	28.00	6.53	19.50	28.50	12.30
July	37.40	24.10	37.90	7.20	19.50	28.20	12.20
August	37.50	24.10	36.80	7.10	19.50	27.10	11.90
September	35.00	21.60	43.50	7.01	21.70	23.70	10.80
October	32.80	17.70	49.50	5.60	19.20	19.90	10.00
November	27.00	12.70	54.70	4.62	15.10	16.50	9.40
<b>Season of 2017</b>							
May	36.30	20.00	31.40	6.56	16.20	27.00	11.40
June	37.40	23.40	34.60	6.67	21.00	28.50	12.30
July	39.10	25.30	32.70	6.76	16.30	28.20	12.20
August	37.80	24.60	38.80	7.42	17.60	27.10	11.90
September	35.30	20.90	44.60	5.87	20.70	23.70	10.80
October	30.30	16.50	47.00	4.86	17.20	19.90	10.00
November	25.10	10.90	54.60	4.49	15.20	16.50	9.40
<b>Season of 2018</b>							
May	37.70	21.70	29.20	6.38	17.50	27.00	11.40
June	38.50	23.20	33.60	6.68	20.00	28.50	12.30
July	38.00	24.70	41.50	7.05	18.70	28.20	12.20
August	37.60	24.30	40.70	7.77	19.80	27.10	11.90
September	35.50	22.00	46.20	6.02	20.50	23.70	10.80
October	32.60	18.90	46.50	5.52	18.10	19.90	10.00
November	26.50	13.10	53.80	4.98	14.70	16.50	9.40
<b>Continue.....</b>	<b>Season of 2019</b>						

Continue.....		Season of 2019					
May	38.10	22.00	28.90	6.56	18.90	27.00	11.40
June	39.00	24.90	33.90	6.89	20.30	28.50	12.30
July	39.10	25.30	32.70	7.41	16.30	28.20	12.20
August	37.80	24.60	38.80	7.99	17.60	27.10	11.90
September	35.30	20.90	44.60	6.45	20.70	23.70	10.80
October	30.30	16.50	47.00	5.71	17.20	19.90	10.00
November	25.10	10.90	54.60	4.84	15.20	16.50	9.40

**Table 5.** Seasonal cropping pattern in Abnoub cropped area

Season	Crop	Area (feddan)					Sum	Average	Average %
		2015 : 2016	2016 : 2017	2017 : 2018	2018 : 2019	2019 : 2020			
Winter	Wheat	23500	21500	24200	18900	28590	116690	23338	60.90
	Clover	11750	11500	10500	13200	10100	57050	11410	29.80
	Bean	—	1000	9550	3915	3425	17890	3578	9.30
	<b>Summation</b>	<b>35250</b>	<b>34000</b>	<b>44250</b>	<b>36015</b>	<b>42115</b>	<b>191630</b>	<b>38326</b>	
Summer	Y- corn	9500	21000	20000	19570	15500	85570	17114	52.40
	Sorghum	13975	8000	10000	11100	10700	53775	10755	32.90
	Basil	2000	6250	5250	4445	5968	23913	4782.6	14.60
	<b>Summation</b>	<b>25475</b>	<b>35250</b>	<b>35250</b>	<b>35115</b>	<b>32168</b>	<b>163258</b>	<b>32651.6</b>	

**Table 6.** Areas of winter and summer crops in Almanna off-taking canals.

No	Canal	Area (fedd)	Areas of winter crops (A <sub>wi</sub> )			Areas of summer crops (A <sub>su</sub> )		
			Wheat 60.9%	Clover 29.8%	Bean 9.3%	Y-corn 52.4%	Sorghum 32.9%	Basil 14.6%
1	Right Almanna side canal	1000	609	298	93	524	329	146
2	Al Hammam	3400	2070.6	1013.2	316.2	1781.6	1118.6	496.4
2--1	Arab meter branch	600	365.4	178.8	55.8	314.4	197.4	87.6
2--2	Al Atawla	500	304.5	149	46.5	262	164.5	73
2--3	Al Zafran	400	243.6	119.2	37.2	209.6	131.6	58.4
2--4	Al Taweel	500	304.5	149	46.5	262	164.5	73
3	Shew	450	274.05	134.1	41.85	235.8	148.05	65.7
4	Western Al Awamer	200	121.8	59.6	18.6	104.8	65.8	29.2
5	Eastern	400	243.6	119.2	37.2	209.6	131.6	58.4

	Alawamer							
6	Northern Alawamer	780	475.02	232.44	72.54	408.72	256.62	113.88
7	Der Shew	400	243.6	119.2	37.2	209.6	131.6	58.4
8	Eastern Al kadadeeh	400	243.6	119.2	37.2	209.6	131.6	58.4
9	Al Hager	500	304.5	149	46.5	262	164.5	73
10	Bani Ibraheem	800	487.2	238.4	74.4	419.2	263.2	116.8
10--1	Bani Ibraheem branch	400	243.6	119.2	37.2	209.6	131.6	58.4
11	Southern Al Gabrawe	500	304.5	149	46.5	262	164.5	73
12	Northern Al Gabrawe	400	243.6	119.2	37.2	209.6	131.6	58.4
13	Alswalem	700	426.3	208.6	65.1	366.8	230.3	102.2
14	Alam Aldeen	500	304.5	149	46.5	262	164.5	73
15	Al Haraga	500	304.5	149	46.5	262	164.5	73

## 2.2 Crop water requirements

CROPWATER software was used for estimating reference evapotranspiration ( $ET_0$  (mm/day)) by using the available meteorological data at Arab Alawamer local weather station as shown in Tables (3 and 4), as shown in Fig. (3) [25]. This program based on the Penman-Monteith method.

Country	2016		Station	2016			
Altitude	48 m.	Latitude	27.00 °N	Longitude	31.00 °E		
Month	Min Temp °C	Max Temp °C	Humidity %	Wind km/day	Sunshine hours	Radiation MJ/m <sup>2</sup> /day	ET <sub>0</sub> mm/day
January	5.1	19.0	60	16	8.9	15.4	1.52
February	8.3	24.5	50	16	9.7	18.7	2.28
March	13.1	28.0	41	19	9.9	21.7	3.21
April	17.1	35.1	31	19	10.3	24.5	4.14
May	20.0	36.1	27	17	11.4	27.0	4.67
June	24.6	40.7	28	13	12.3	28.5	5.22
July	24.1	37.4	37	8	12.2	28.2	5.16
August	24.1	37.5	36	8	11.9	27.1	4.82
September	21.6	35.0	43	14	10.8	23.7	4.09
October	17.7	32.8	49	16	10.0	19.9	3.09
November	12.7	27.0	54	15	9.4	16.5	2.02
December	6.3	19.9	59	16	9.0	14.8	1.41
Average	16.2	31.1	43	15	10.5	22.2	3.47

Fig. 3. Computer windows of CROPWATER software program.

**2.3 Actual crop evapotranspiration**

Evapotranspiration from a cropped field is composed of transpiration from the crop and evaporation from the soil. The rate of evapotranspiration from the crop ( $ET_c$ ) or crop water use depends on the type of crop, stage of growth, moisture content of the surface soil, and the amount of energy available to evaporate water. Crop water use ( $ET_c$ ) is computed using the reference crop evapotranspiration ( $ET_o$ ) and a crop coefficient ( $K_a$ ) as follow:

$$ET_c = K_a \times ET_o \dots\dots\dots (2)$$

The crop coefficient ( $K_a$ ) depends on the growth and development of the crop canopy. It must include the basal crop coefficient ( $K_{cb}$ ) and the effect of wet soil evaporation ( $K_w$ ). If water stress is expected, an appropriate stress factor ( $K_s$ ) can also be selected although this is generally not carried out [26 & 27]. The average crop coefficient ( $K_a$ ) is estimated as:

$$K_a = (K_{cb} \times K_s) + K_w \dots\dots\dots (3)$$

$K_w$  = factor to account for increase evaporation (wet soil evaporation factor) [29].

$$K_w = F_w \times (1 - K_{cb}) \times A_f \dots\dots\dots (4)$$

where  $F_w$  = the fraction of the soil surface wetted that take 1.0 for basin irrigation and  $A_f$  = the average wet soil evaporation factor as reported by Paul et al. [27]. The crop coefficient system developed by Doorenbos and Pruitt [30] and modified by Howell et al. [31] to estimate actual crop evapotranspiration. To use this method, the growing season is divided into four stages, as shown in Fig. (4):

**Initial:** Period from planting through early growth when the soil is not, or is hardly, covered by the crop (ground cover <10%).

**Canopy development:** Period from initial stage to the time that the crop effectively covers the soil surface (ground cover about 70 to 80%).

**Mid-season:** Period from full cover until the start of maturation when leaves begin to change color or senesce.

**Maturation:** Period from end of mid-season until physiological maturity or harvest.

The progression of the basal crop coefficient during the season is illustrated in Fig. (4) for Yellow corn as an example.

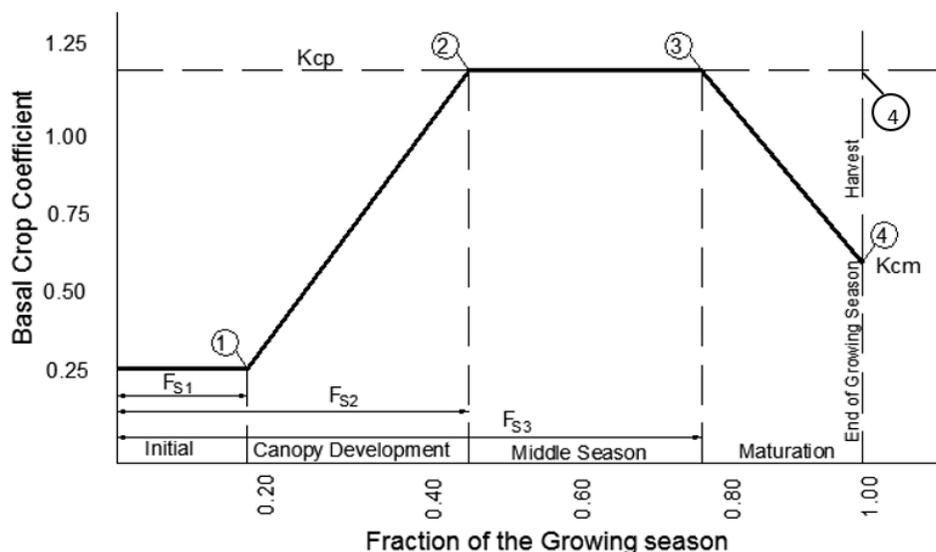
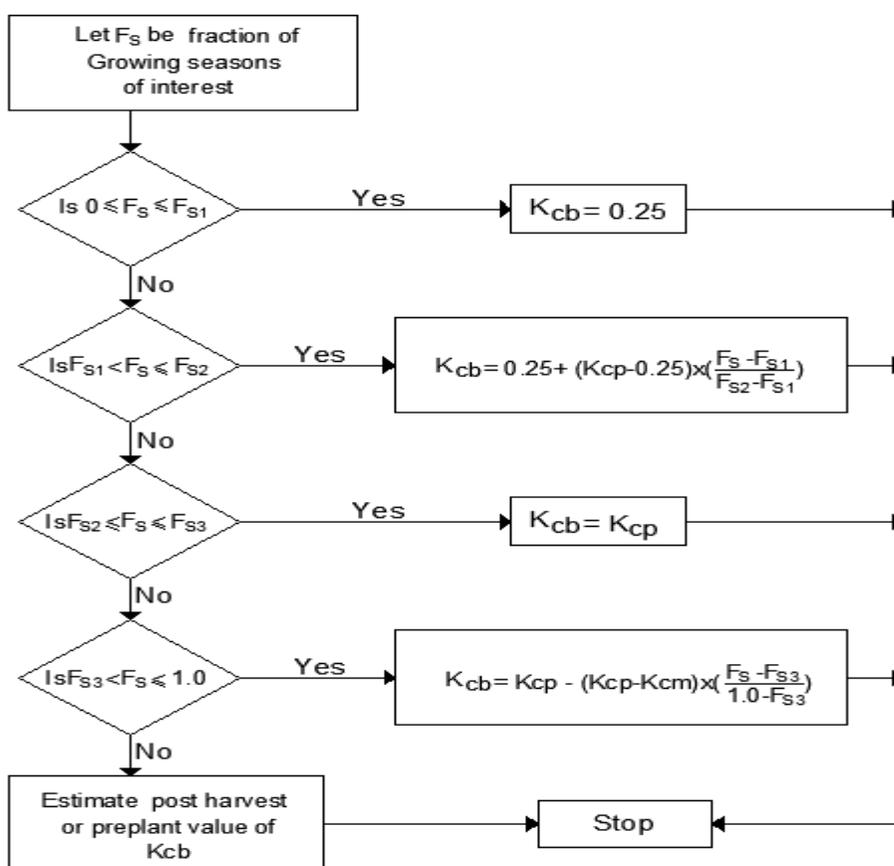


Fig. 4. Basal crop coefficient ( $K_{cb}$ ), [27].

To compute the crop coefficient during other periods of crop development, four points on the crop coefficient curve need to be defined. The first point is the fraction of the growing season where canopy development begins (point 1 in Fig. 4). At this point, the value of  $K_{cb}$  (0.25) is known based on the assumption in the preceding paragraph, so only  $F_{S1}$  is needed. The second point occurs when the canopy has developed adequately to provide effective cover. Currently, the basal crop coefficient reaches its peak value. Thus, for the second point (point 2 in Fig. 4), both the peak values of  $K_{cb}$  ( $K_{cp}$ ) and  $F_{S2}$  are needed. Point 3 in Fig. 4 is the time when the crop begins to mature. The only value needed for the third point is the time ( $F_{S3}$ ) because the crop coefficient at point 3 equals the peak value of the basal crop coefficient. For the fourth point, two locations are shown in Fig. (4). The lower location represents crops that begin to senesce before harvest. To define this point, the value of the basal crop coefficient at maturity ( $K_{cm}$ ) must be known. If the crop is harvested before the plant begins to mature, the crop coefficient remains constant at the peak value until harvest (the second location of point 4 in Fig. 4). Hence, the needed five definitions to compute the crop coefficient ( $F_{S1}$ ,  $F_{S2}$ ,  $F_{S3}$ ,  $K_{cp}$ ,  $K_{cm}$ ) are labeled in Fig. 4. Values for the five parameters needed to compute the basal crop coefficients for different crops in the study area are summarized in Table (7) [27]. The procedure to compute the basal crop coefficient for any stage of growth is plotted in Fig. (5). Also, Table (8) shows the calculated values of the monthly average crop coefficient ( $K_a$ ) for the different crops in the study area.

**Table 7.** Values of ( $F_{S1}$ ,  $F_{S2}$ ,  $F_{S3}$ ,  $K_{cp}$ ,  $K_{cm}$ ) for different crops, [27]

Crop	$F_{S1}$	$F_{S2}$	$F_{S3}$	$K_{cp}$	$K_{cm}$
Wheat	0.13	0.33	0.75	1.05	0.25
Bean	0.16	0.42	0.80	1.15	0.25
Clover	0.20	0.40	0.90	1.10	1.0
Yellow corn	0.17	0.45	0.78	1.15	0.60
Sorghum	0.16	0.42	0.75	1.10	0.55
Basil	0.86	1	1	0.07	0.93

**Fig. 5.** Process to compute basal crop coefficient ( $K_{cb}$ ), [27].

**Table 8.** Values of the monthly average crop coefficient ( $K_a$ ) for different crops.

Crop	Month	Days of month	Day of growing	Time since planting	$F_s$	$K_{cb}$	$K_s$	$F_w$	$A_f$	$K_w$	$K_a$
Wheat	November	30	15	15	0.09	0.25	1	1	0.511	0.38	0.63
	December	31	31	46	0.28	0.84	1	1	0.511	0.08	0.92
	January	31	31	77	0.46	1.05	1	1	0.511		1.05
	February	28	28	105	0.63	1.05	1	1	0.511		1.05
	March	31	31	136	0.82	0.83	1	1	0.511	0.09	0.92
	April	30	30	166	1.00	0.25	1	1	0.511	0.38	0.63
Bean	October	31	15	15	0.10	0.25	1	1	0.511	0.38	0.63
	November	30	30	45	0.30	0.73	1	1	0.511	0.14	0.87
	December	31	31	76	0.51	1.15	1	1	0.511		1.15
	January	31	31	107	0.71	1.15	1	1	0.511		1.15
	February	28	28	135	0.90	0.7	1	1	0.511	0.15	0.85
	March	31	15	150	1.00	0.25	1	1	0.511	0.38	0.63
Clover	October	31	15	15	0.07	0.25	1	1	0.511	0.38	0.63
	November	30	30	45	0.21	0.45	1	1	0.511	0.28	0.73
	December	31	31	76	0.35	0.94	1	1	0.511	0.03	0.97
	January	31	31	107	0.50	1.1	1	1	0.511		1.10
	February	28	28	135	0.63	1.1	1	1	0.511		1.10
	March	31	31	166	0.77	1.1	1	1	0.511		1.10
	April	30	30	196	0.91	1.08	1	1	0.511		1.08
	May	31	20	216	1.00	1	1	1	0.511	0.00	1.00
Basil (ocimum)	May	31	15	15	0.11	0.25	1	1	0.511	0.38	0.63
	June	30	30	45	0.33	0.86	1	1	0.511	0.07	0.93
	July	31	31	76	0.55	1.15	1	1	0.511		1.15
	August	31	31	107	0.78	1.15	1	1	0.511		1.15
	September	30	30	137	1.00	1	1	1	0.511	0.00	1.00
Yellow corn	May	31	15	15	0.10	0.25	1	1	0.511	0.38	0.63
	June	30	30	45	0.30	0.76	1	1	0.511	0.12	0.88
	July	31	31	76	0.51	1.15	1	1	0.511		1.15
	August	31	31	107	0.71	1.15	1	1	0.511		1.15
	September	28	28	135	0.90	0.6	1	1	0.511	0.20	0.80
Sorghum	May	31	15	15	0.11	0.25	1	1	0.511	0.38	0.63
	June	30	30	45	0.33	0.29	1	1	0.511	0.36	0.65
	July	31	31	76	0.55	1.1	1	1	0.511		1.10
	August	31	31	107	0.78	1.1	1	1	0.511		1.10
	September	30	30	137	1.00	0.55	1	1	0.511	0.23	0.78

**2.4 Canal discharge computation**

Water consumption ( $WC$ ) in ( $m^3/sec$ ) needed for the areas served by different distributary canal in the study area is computed using the approach introduced by El-Enany et al. [32] which uses the following Eqn.:

$$WC = \left( \frac{ET_0 K_a A_{ci}}{1000} \right) \times \frac{4200}{86400} \dots\dots\dots (5)$$

where  $ET_o$  is the monthly average evapotranspiration for the area served by a distributary canal (mm/day),  $K_a$  is monthly average crop coefficient, and  $A_{ci}$  is the area cultivated (feddan).

Table (9) shows the water consumption needed for the cultivated crops in the area served by Al Hammam distributary canal every month as an example. From this table, it is observed that, water consumption values of crops are different during the growth stages. As it is low at the beginning of cultivation (initial stage) and gradually increases to reach its highest value in stage of mid-season, then it returns to decline again in a maturation stage as shown previously in Fig. (4). Also, the last column in Table (9) shows the monthly total values of water consumption of crops grown in this area. It is noticed that the highest value of water consumption occurs in July, while the lowest value occurs in October, which is the beginning of the cultivation of winter crops and the end of the harvest of summer crops.

Hence, it turns out that large quantities of irrigation water can be provided when water is given to crops according to their water needs in the different growth stages and not at fixed values throughout the year. The discharges of the different distributary canals in the study area corresponding to the water consumption needed can be estimated by Eq. (6) [32].

$$Q_d = Wc \times \frac{N_1}{N_2} \times \left( \frac{24}{T_e - T_s} \right) \times (1 + L_R) / \eta \dots\dots\dots (6)$$

where  $Q_d$  is the discharge of distributary canal during hours of irrigation ( $m^3/sec.$ ),  $T_e$  is the assumed day hour at which irrigation ends,  $T_s$  is the assumed day hour at which irrigation starts,  $L_R$  is the leaching requirement, which is a ratio of water consumptive use (%)  $L_R=0.05$ , and  $\eta$  is the irrigation efficiency for distributary canal, where conveyance and on-farm efficiencies are included (= 0.71) [32].

The difference between design and new discharges were calculated at each month to estimate the amount of water saved. Table (10) shows the value of design and new discharges of Al Hammam distributary canal during different months, as an example.



**Table 10.** Values of designed, new, and saved discharge Al Hammam distributary canal during month in (Q) m<sup>3</sup>/sec, as an example

Month	Needed Discharge (Q <sub>c</sub> )	Bed width (B)	Needed water depth (Y)	Water slope (S)	Side slope (Z)	1/n	The lowest water depth that can be used (Y)	The discharge corresponding to the lowest water depth (Q)	Designed discharge (Q <sub>des</sub> )	The value of saved discharge (Q)
January	0.86	4	0.62	0.00005	1:1	67	0.90	1.61	4.39	2.78
February	1.25	4	0.78	0.00005	1:1	67	0.90	1.61	4.39	2.78
March	1.55	4	0.88	0.00005	1:1	67	0.90	1.61	4.39	2.78
April	1.47	4	0.46	0.00005	1:1	67	0.90	1.61	4.39	2.78
May	2.17	4	1.07	0.00005	1:1	67	1.07	2.17	4.39	2.22
May	2.22	4	1.09	0.00005	1:1	67	1.09	2.22	4.39	2.17
June	3.09	4	1.31	0.00005	1:1	67	1.31	3.09	4.39	1.30
July	2.92	4	1.27	0.00005	1:1	67	1.27	2.92	4.39	1.47
August	1.82	4	0.97	0.00005	1:1	67	0.97	1.83	4.39	2.56
October	0.40	4	0.39	0.00005	1:1	67	0.90	1.61	4.39	2.78
November	0.68	4	0.54	0.00005	1:1	67	0.90	1.61	4.39	2.78
December	0.73	4	0.54	0.00005	1:1	67	0.90	1.63	5.39	2.78

### 3. Results and Discussion:

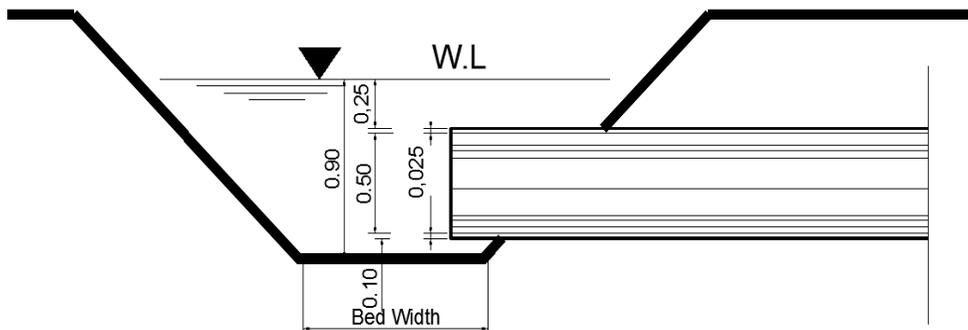
The required monthly discharge was calculated for each canal in the study area according to the water needs of the crops cultivated in that month. Also, water depth corresponding to each monthly discharge was calculated using Manning equation, considering that the canal bed width, the side slopes, and the longitudinal slope as constants. As shown in Fig. (6), a minimum water depth of 0.90 m in canals has been suggested if the required depth is less than this value. This value was suggested for the water depth to be the lowest water height above the inlet pipe of any field by about 25 cm to ensure that the water reaches these fields in the required quantities.

Table (11) shows the values of the designed, new, and saving discharges in all Almana distributary canals. From this table, it is clear that:

- In most canals, values of the new discharges varied between 32 and 67% of the designed discharges.
- For Der Shew, and Al Hager canals, the value of the required discharge equal to the designed discharge.
- The total value of discharge saved from Almana distributary canals equals to 14.95 m<sup>3</sup>/sec, representing 48.68% of all irrigation water given to Almana distributary canals.

**Table 11.** Value of designed, new, and saved discharge of Almana off-taking canals

No	Canal	Area (Fedd)	Bed Width	Q <sub>Designed</sub> (m <sup>3</sup> /sec)	Q <sub>new</sub> (m <sup>3</sup> /sec)	Q <sub>saved</sub> (m <sup>3</sup> /sec)
1	R. Almana s. canal	1000	2.0	2.05	<b>1.21</b>	<b>0.84</b>
2	Al Hammam	3400	4.0	4.39	<b>1.61:3.09</b>	<b>1.30:2.78</b>
2-1	Arab meter branch	600	2.0	1.45	<b>0.85</b>	<b>0.60</b>
2-2	Al Atawla	500	1.5	1.65	<b>0.95</b>	<b>0.70</b>
2-3	Al Zafran	400	1.5	1.34	<b>0.70</b>	<b>0.64</b>
2-4	Al Taweel	500	1.5	2.92	<b>0.99</b>	<b>1.93</b>
3	Shew	450	1.0	1.26	<b>0.88</b>	<b>0.38</b>
4	Western Al Awamer	200	1.0	1.38	<b>0.62</b>	<b>0.76</b>
5	Eastern Alawamer	400	1.0	0.94	<b>0.62</b>	<b>0.32</b>
6	Northern Alawamer	780	1.5	2.05	<b>0.67</b>	<b>1.38</b>
7	Der Shew	400	1.0	0.50	<b>0.50</b>	<b>0.00</b>
8	Eastern Al kadadeeh	400	1.0	0.74	<b>0.50</b>	<b>0.24</b>
9	Al Hager	500	1.5	0.95	<b>0.95</b>	<b>0.00</b>
10	Bani Ibraheem	800	2.0	1.74	<b>0.85</b>	<b>0.89</b>
10-1	Bani Ibraheem bran.	400	1.0	1.45	<b>0.50</b>	<b>0.95</b>
11	Southern Al Gabrawe	500	1.0	2.01	<b>0.70</b>	<b>1.31</b>
12	No.n Al Gabrawe	400	1.0	0.81	<b>0.50</b>	<b>0.31</b>
13	Alswalem	700	1.5	1.93	<b>0.95</b>	<b>0.98</b>
14	Alam Aldeen	500	1.0	0.90	<b>0.50</b>	<b>0.40</b>
15	Al Haraga	500	1.0	1.15	<b>0.70</b>	<b>0.45</b>



**Fig. 6.** Modified cross section of the canal

#### 4. Conclusions

The study introduced and applied a new approach for water allocation scheduling in irrigation open channels. This approach is mainly depending on the good estimation of crop water needs in the each of plant growth stage. The main conclusions can be summarized as follows:

- [1] Applying the introduced approach may lead to save a large amount of irrigation water.
- [2] According to the total monthly values of water consumption of crops grown in the study area, the highest value of water consumption was in July, while the lowest value was in October, so the delivery flow rates should be scheduled accordingly.
- [3] The percentage value of the new discharges of Almanna distributary canals, which calculated according to the water needs of crops cultivated in the area, varied between 32% and 67% of the design discharges.
- [4] Applying the introduced approach on Almanna distributary canals can be saving a large amount of irrigation water that reaches about 19.375 Million cubic meters per month, which is representing 48.68% of all irrigation water given to Almanna distributary canals.

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## أسلوب جديد لجدولة توزيع المياه في قنوات الري المفتوحة حالة دراسة: ترعة المعنا الرئيسية وفروعها.

في مصر، يستهلك قطاع الزراعة حوالي ٨٥,٩٠٪ من إجمالي الاستهلاك الفعلي للمياه. في حين أن شبكة قنوات الري في مصر على جميع المستويات تعاني من فقد المياه، والقطاعات المشوهة، والتوزيع غير العادل لمياه الري بين المستفيدين. ومع النمو السكاني والتوسع في الأنشطة الاقتصادية والصناعية ودخول مصر إلى مرحلة الفقر المائي، أصبح ترشيد استهلاك مياه الري مطلباً عاجلاً وعملاً رئيسياً في تحقيق التنمية المائية المستدامة. وفقاً لنظام الري المتبع في مصر فإنه يتم تزويد شبكة قنوات الري خلال العام بنفس التصريف الشهري الثابت سواء خلال فترات الحد الأقصى أو الحد الأدنى من الاحتياجات المائية للنبات. على الرغم من أنه قد يحتاج إلى أكثر أو أقل من هذه الكميات خلال مراحل النمو المختلفة. الهدف من هذه الدراسة هو تفعيل أسلوب جديد لإدارة شبكة قنوات الري لمنطقة الدراسة. يسمح هذا الأسلوب بتزويد شبكة القنوات بمعدلات التصريف الشهرية المتوافقة مع استهلاك المياه الفعلي للمزروعات. وتساعد هذه الطريقة في ترشيد الاستهلاك خلال الموسم الزراعي. تم تطبيق الطريقة على قناة المعنا الرئيسية وفروعها التابعة لمحافظة أسيوط في وسط مصر، كمثال لنظام الري في مصر حيث يمارس التوزيع الدوراني على مستوى قناة التوزيع. تشير النتائج إلى أن التقدير الجيد لاحتياجات المحاصيل المائية في كل مرحلة من مراحل النمو قد يؤدي إلى توفير كمية كبيرة من مياه الري تقدر بـ ١٩,٣٧٥ مليون متر مكعب شهرياً، تمثل ٤٨,٦٨٪ من إجمالي مياه الري التي تعطى لقنوات المعنا التوزيعية.